HARNESS RACING NSW





FLOOD IMPACT ASSESSMENT FOR PROPOSED HARNESS RACING TRACK AT NORTH WAGGA WAGGA

FINAL





AUGUST 2017





Level 2, 160 Clarence Street Sydney, NSW, 2000

Tel: (02) 9299 2855 Fax: (02) 9262 6208 Email: wma@wmawater.com.au Web: www.wmawater.com.au

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Client		Client's Representative	
Harness Racing NSW		John Dumesny	
Authors		Prepared by	
Kieran Smith		18 1	
Erin Askew		280mmly	
Date		Verified by	
18 August 2017		John	
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LIST OF ACRONYMS

AEP	Annual Exceedance Probability		
ARI	Average Recurrence Interval		
ALS	Airborne Laser Scanning		
ARR	Australian Rainfall and Runoff		
BOM	Bureau of Meteorology		
DECC	Department of Environment and Climate Change (now OEH)		
DNR	Department of Natural Resources (now OEH)		
DRM	Direct Rainfall Method		
DTM	Digital Terrain Model		
GIS	Geographic Information System		
GPS	Global Positioning System		
IFD	Intensity, Frequency and Duration (Rainfall)		
mAHD	meters above Australian Height Datum		
OEH	Office of Environment and Heritage		
PMF	Probable Maximum Flood		
SRMT	Shuttle Radar Mission Topography		
TUFLOW	One-dimensional (1D) and two-dimensional (2D) flood and tide		
	simulation software (hydraulic model)		
WBNM	Watershed Bounded Network Model (hydrologic model)		

1. INTRODUCTION

WMAwater were engaged by Harness Racing NSW (HRNSW) to provide a flood impact assessment for the proposed construction of a Harness Racing Track and associated amenities and facilities at Cooramin Street, Cartwrights Hill. This report details the process undertaken to determine the hydraulic impacts of such works on both riverine and local catchment flooding for the 10%, 5%, 2% and 1% AEP events.

2. BACKGROUND

HRNSW is proposing the construction of a new Harness Racing Track and associated amenities and facilities on an undeveloped parcel of land bounded by Hampden Avenue, Cooramin Street and Wright Street in Cartwrights Hill. This area has been zoned as Rural Primary Production (RU1) as per Wagga Wagga City Council (WWCC) LEP 2010. The proposed development involves the construction of a new harness racing track, trotting track and associated infrastructure including (but not limited to):

- Track, trailer and car parking;
- Stables;
- A clubhouse;
- Fencing; and
- A detention basin.

The area where development is to take place is flood prone in the 5-year (20% AEP) riverine event, with a peak flood depth of 0.61 m along the southern boundary and flooding across most of the proposed site extent (see Figure 1). As such a flood impact assessment is required to accompany the development application (DA.). The area is impacted by both riverine flooding from the Murrumbidgee River and local catchment flooding from the Duke's Creek catchment. A hydraulic assessment of the proposed development will be undertaken utilising hydraulic models of both flood mechanisms to determine the impacts it will have on riverine and local catchment flooding behaviour.

WMAwater have previously undertaken a flood impact assessment for the proposed harness racing track construction in July 2013, May 2014 and September 2015. Those assessments dealt only with the impact of the proposed development on riverine flooding and not local catchment flooding. Additionally, the previous work assumed that the Wagga Wagga Main City Levee was in its upgraded state. Construction is due to commence as this report is being prepared. Water NSW have requested that the impacts be considered with the Main City Levee in its current state.

3. METHODOLOGY

3.1. Model Overview

3.1.1. Riverine Flood Model

The Murrumbidgee River TUFLOW 1D/2D hydraulic model developed by WMAwater (2017) has been utilised in carrying out the riverine flood impact assessment. The model extends from approximately 5 km upstream of Oura to 9 km downstream of Malebo Gap, giving a total river reach of approximately 63 km. This is the model version currently being used as part of the Wagga Wagga Riverine Floodplain Risk Management Study.

The model is defined by a 20 m by 20 m finite difference grid. The model's ground elevation has been informed by ALS data recorded in 2008 by Fugro Spatial Systems Pty Ltd for the entire Murrumbidgee River floodplain. River bathymetry survey was undertaken by a qualified hydrosurvey firm who produced a dataset of cross-sections which were utilised to create a DEM of the riverbed.

Design flows have been developed via Flood Frequency Analysis (FFA). The FFA was undertaken as part of the Wagga Wagga Detailed Flood Model Revision (2014) and used an annual maximum series obtained for the Hampden Bridge gauge at Wagga Wagga (No. 410001.) Calibration and validation of the hydraulic model has been carried out utilising flood intelligence information collected for the 2010 and 2012 flood events.

The riverine hydraulic model setup is shown on Figure 2.

3.1.2. Local Catchment Flood Model

The Wagga Wagga Overland Flow TUFLOW 1D/2D hydraulic model developed by WMAwater (2015) has been utilised in carrying out the local catchment flood impact assessment. The model provides detailed Major Overland Flow design flood information for an area of approximately 270 km². The model includes the North Wagga model domain which covers the Duke's Creek catchment.

The model is defined by a 5 m by 5 m finite difference grid. Flows were determined from AR&R 1987 design storms. Calibration and validation of the hydraulic model has been carried out utilising flood intelligence information collected for the February and December 2010 flood events.

The local catchment hydraulic model setup is shown on Figure 3.

3.2. Model Modifications

Design level information for the proposed racetrack and associated amenities have been provided by McKinnon design (Proposed Wagga Harness Racing Club Facility, 20/3/2017 see Appendix A). The information can be summarised as follows:

- **Existing Ground Level Survey:** Detailed survey of the existing lots has been provided for lot DP 2655. This can be seen in Figure 4.
- **Detailed Design Survey:** Detailed survey of the proposed track design and surrounding area has been provided.
- **Public Stand:** Extent and position information for the proposed Public Stand.
- Stables: Extent and position information for the proposed stables.
- **Fencing:** Detailed design information for the proposed track fencing.

Figure 5 shows design plans and levels for the proposed racetrack and infrastructure. Detailed design drawings are also included as Appendix A.

Existing Ground Levels at the site, detailed design survey and infrastructure information have been incorporated into the hydraulic model as following:

- Existing Ground Level Survey was incorporated into both the existing and proposed riverine and local catchment flood models as a 3D tin, as it was considered to be a more accurate representation of ground levels in the area than the ALS data used in the existing models;
- Detailed Design Survey for the developed case was incorporated into the proposed riverine and local catchment flood models as a 3D tin;
- The public stand and stables were incorporated into the proposed riverine and local catchment flood models as 2D terrain modifiers set at 181.8 mAHD and 179.6 mAHD respectively based on the design height of the buildings;
- The fencing was incorporated into the proposed riverine and local catchment flood models as a 2D flow constriction in order to impede flow onto the track. A detailed description of the fencing set-up is included as Appendix B.

3.3. Design Runs

3.3.1. Riverine Flood Model Design Runs

The riverine flood model was re-run with the existing ground level survey incorporated for the 20%, 10%, 5%, 2% and 1% AEP design events. The model was then modified to include the detailed design survey and associated amenities described above and the proposed case was run for the 10%, 5%, 2% and 1% AEP design events. Peak riverine flood level, velocity and hazard impacts are discussed in Section 4.1 below.

3.3.2. Local catchment Flood Model Design Runs

The local catchment flood model was re-run with the existing ground level survey incorporated for the 10%, 5%, 2% and 1% AEP design events. The model was then modified to include the detailed design survey and associated amenities described above and the proposed case was run for the 10%, 5%, 2% and 1% AEP design events. In both the existing and design cases a downstream boundary condition independent of riverine flooding (i.e. it was assumed there was no coincident local catchment and riverine flooding) was utilised.

Peak local catchment flood level, velocity and hazard impacts are discussed in Section 4.2 below.

4. RESULTS

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4.1. Riverine Flood Impacts

Results for Riverine Flooding are presented in Figure 6 to Figure 17.

4.1.1. Peak Flood Level Impact

In the 10%, 5%, 2% and 1% AEP flood events the proposed racetrack and infrastructure cause a maximum peak flood level increase of 0.02 m in open areas adjacent to the racetrack precinct (Figure 6 to Figure 9). Peak flood level decreases are also seen in the rarer events at Horsehoe Road with a peak decrease of 0.16 m shown in the 2% AEP event.

4.1.2. Peak Flood Velocity Impact

In the 10% AEP flood event minimal change in peak flood velocity is observed outside the racetrack precinct (Figure 10). Isolated changes can be observed between -0.25 to 0.25 m/s. In the 5%, 2% and 1% AEP events increases in peak flood velocity of up to and above 0.5 m/s can be seen along Cooramin Road to the south (Figure 10 to Figure 12) as flows from the west are newly diverted around the proposed track.

4.1.3. Peak Flood Hazard Impact

For the range of design events investigated current provisional hydraulic hazard at the racetrack precinct and adjacent properties is identified as High Hazard. Figure 14 to Figure 17 show the change in provisional hydraulic hazard after proposed racetrack works. Modelling results indicate change in provisional hydraulic hazard from High to Low generally localised at the racetrack perimeter as a result of the change in flood depth due to filling as a result of the proposed development. This change occurs in all design events except for the 1% AEP. Isolated patches of hazard increase are located to the north-east of the proposed track for all design events.

4.2. Local Catchment Flood Impacts

Results for Local Catchment Flooding are presented in Figure 18 to Figure 29

4.2.1. Peak Flood Level Impact

Increases to local catchment peak flood levels are observed for the range of design events investigated, with impacts scaled through the rarer events. In the 10% AEP event (Figure 18), peak flood levels are increased to the west of the proposed site and in the basin area to the south west (along with newly flooded areas within the basin.) This behaviour is also observed in the 5%, 2% and 1% AEP events (Figure 19 to Figure 21) with a peak increase of 0.17 m in the 1% event within the basin area. The proposed construction also causes decreases in flood levels to the south of the site up to 0.3 m in the 1% event.

4.2.2. Peak Flood Velocity Impact

Peak flood velocities are increased to the west and south-west of the proposed development up to 0.1 m/s across all design events (Figure 22 to Figure 25.) The largest changes to peak flood velocity are observed at the northern and southern boundaries of the proposed development. In the 1% AEP event increases in peak flood velocity of up to and above 0.5 m/s in isolated locations can be seen along Cooramin Road to the south of the proposed racetrack.

4.2.3. Peak Flood Hazard Impact

For the range of design events investigated current provisional hydraulic hazard at the racetrack precinct and adjacent properties is identified as Low hazard. Figure 26 to Figure 29 show the change in provisional hydraulic hazard after proposed racetrack works. Modelling results indicate isolated change in provisional hydraulic hazard from Low to High to the west and south-west of the proposed racetrack across all design events. Minor isolated increases are observed in the 10% and 5% AEP Events, with larger increases (localised mostly within the basin to the south-west) observed in the 2% and 1% AEP Events.

4.3. Flood Warning Time and Rate of Rise

4.3.1. Riverine

Murrumbidgee River flooding at Wagga Wagga primarily originates in the upper areas of the Murrumbidgee River catchment with releases from Burrinjuck and Blowering dams. Rainfall downstream of Burrinjuck Dam generally provides a relatively modest contribution to flow. The Erringoarrah stream gauge is the next gauge upstream of Hampden Bridge. It provides Wagga Wagga with sufficient warning time during flood events for evacuation purposes. The last flood in 2012, which was estimated to be a 3% AEP event, took approximately 21 hours to travel from Erringoarrah to Wagga Wagga. Note that flood travel time is indirectly correlated to flood size (i.e. the larger the flood, the shorter the time it takes the flood wave to arrive).

For design events, current travel time of flooding from Oura to North Wagga are as following:

- 10% AEP: 24 h
- 5% AEP: 21 h
- 2% AEP: 15 h
- 1% AEP: 12 h

Modelling results indicate the construction of the racetrack will not affect flood warning time.

The rate of rise describes the average increase in water level, in meters per hour, at a given location (see Figure 5). Current rate of rise values within the racetrack precinct are presented below:

- 10% AEP: 0.17 m/h
- 5% AEP: 0.14 m/h
- 2% AEP: 0.12 m/h
- 1% AEP: 0.17 m/h

Modelling results indicate that construction of the racetrack will not affect rate of rise of riverine floodwaters.

4.3.2. Local Catchment

Local Catchment flooding into Dukes Creek originates from multiple areas North of Wagga Wagga: from Boorooma directly to the north and Bomen to the north-east. Local catchment flooding is primarily driven by rainfall in the local catchments as opposed to riverine flooding and subsequently has a much shorter response time. In all design events, the proposed site has a relatively short first flooded time of around 45 minutes and is completely inundated within 3 hours. Modelling results indicate the construction of the racetrack will not affect flood warning time.

The rate of rise describes the average increase in water level, in meters per hour, at a given location (see Figure 5). Current rate of rise values within the racetrack precinct are presented below:

- 10% AEP: 0.09 m/h
- 5% AEP: 0.08 m/h
- 2% AEP: 0.05 m/h

• 1% AEP: 0.07 m/h

Modelling results indicate that construction of the racetrack will affect rate of rise of floodwaters at the site, particularly in rarer events. Estimated rate of rise values following construction of the racetrack are shown below.

- 10% AEP: 0.15 m/h
- 5% AEP: 0.15 m/h
- 2% AEP: 0.19 m/h
- 1% AEP: 0.37 m/h

4.4. Flood Evacuation Consideration

Flood evacuation considerations for both riverine and local catchment flooding of the proposed site are discussed below.

4.4.1. Riverine Flooding

In all events above the 10% AEP, the proposed track will be inundated by riverine flooding with depths well above 1 m. The proposed track acts as a low flood island in such events, with the track initially surrounded by water and then overtopped at a later stage. In the 10% AEP event, Cooramin Street is cut at 24 hours after the Hampden Gauge level exceeds 9.25m with both Wright Street and Hampden Avenue cut by 27 hours. In the 1% AEP event, all 3 roads are cut by 15 hours after the Hampden Gauge level exceeds 9.75m. These roads remain cut for at least 30 hours.

Long flood warning times are available for riverine flooding of the track (24 hours for a 10% AEP event through to 12 hours for a 1% AEP event) and there is a large amount of flood free land present to the north of the proposed site that is suitable for evacuation. Appropriate operational planning at the site would be required during a flood event.

Road access to the Horseshoe Road Motor Village Caravan Park is available via Hampden Street for the duration of the riverine 1% AEP flood event.

4.4.2. Local Catchment Flooding

The short inundation time of the proposed track site means that there is little flood warning time for potential evacuation. In the 1% AEP event, access to the site from both Cooramin Street and Wright Street is cut by 90 minutes into the flood, and although Hampden Avenue remains relatively flood free vehicle access and evacuation to the north may not be possible. However, the proposed track is not expected to be overtopped in any event up to and including the 1% AEP event.

Road access to the Horseshoe Road Motor Village Caravan Park is cut during the local catchment 1% AEP event by 90 minutes into the flood.



5. CONCLUSIONS

WMAwater were engaged by Harness Racing NSW (HRNSW) to provide a flood impact assessment for the proposed construction of a Harness Racing Track and associated amenities and facilities at Cooramin Street, Cartwrights Hill. As per the request of WaterNSW, the assessment comprised individual riverine and local-catchment flood impact assessments and included updates to previous assumptions regarding the design of the track and site, and the height of the Wagga Wagga Main City Levee.

The assessment of riverine flood impacts found that the inclusion of the proposed race track infrastructure (including stable complex, clubhouse and fencing) does not significantly alter flood impacts previously reported with a max increase of 0.02 m in adjacent open areas to the south of the track.

The assessment of local-catchment flood impacts found that the construction of the proposed race track and infrastructure (including stable complex, clubhouse and fencing) will cause increases to peak flood levels outside of the proposed site area, although the majority of this impact is localised in the detention basin to the south-west of the proposed site which has a max increase of 0.17 m.



6. REFERENCES

- WMAwater
 Wagga Wagga Revised Murrumbidgee River Floodplain Risk Management Study and Plan (*Draft – Stage 3*)
 Wagga Wagga City Council, July 2017
- 2. WMAwater

Wagga Wagga Major Overland Flow – Model Update Report Wagga Wagga City Council, November 2015

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Appendix A











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APPENDIX B

Design drawings for the proposed Harness Racing Track (Appendix A, Drawing G6) identifies the proposed fencing to be utilised around the track. The drawings identify two types of fencing to be used:

- Palisade fencing around along the south side of the track; and
- Colorbond Steel fencing around the remainder of the track.

The location of both the palisade fencing (yellow) and the Colorbond Steel fencing (red) are shown in Image 1 below.



Image 1: Location of Fencing

The design of these fences have a different blockage effect on the flow of flood waters. The proposed palisade fence is a semi-open design that is roughly 80% clear. The proposed colorbond steel fence is a solid fence design which is 100% blocked, although there is a small gap underneath the fence which allows some water to flow through.

Both fences have been modelled for this assessment as constrictions. The proposed palisade fence has been modelled as having a constant 1.2 m height with a constant blockage factor of 20%. The colorbond fence has been modelled with a bottom gap (of 0.1 m) that is unblocked before a constant 1.2 m height that is fully blocked. This is shown in Image 2 below.



Image 2: Colorbond Fence Blockage

